

DIAL BEFORE YOU DIG.

CONTRACTOR MUST DIAL 1100 TO CONFIRM LOCATION OF EXISTING SERVICES AND COMPLY WITH ANY AUTHORITY REQUIREMENTS REGARDING EXISTING SERVICES PRIOR TO COMMENCEMENT OF ANY WORKS.

SLAB AND FOOTING NOTES:

- ALL FOOTINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH SOIL REPORT NO.30164-1 PREPARED BY STATEWIDE GEOTECHNICAL DATED 22ND OF NOVEMBER 2023.
- SITE CLASSIFICATION - CLASS "P", HOWEVER FOOTINGS DESIGNED FOR CLASS "M" TYPE SOIL CONDITIONS IN ACCORDANCE WITH AS2870-2011.
- SLAB AND FOOTINGS ARE TO PENETRATE THROUGH ANY SURFACE FILL AND NATURAL SILTY SAND AND BE FOUNDED A MINIMUM OF 100mm INTO THE NATURAL SANDY CLAY WHERE A MINIMUM ALLOWABLE BEARING CAPACITY OF 100kPa EXISTS OR AT LEAST 800mm BELOW GROUND LEVEL, WHICHEVER IS DEEPER.
- IT IS THE BUILDERS RESPONSIBILITY TO ENSURE THAT THE ANGLE OF REPOSE (CLAY=45°, SAND=30°) MISSES THE BOTTOM OF ALL PIPES.
- FOOTINGS WITHIN THE ZONE OF INFLUENCE OF TREES ARE TO BE DEEPENED DOWN TO ROCK OR AT LEAST 1500mm BELOW GROUND LEVEL, WHICHEVER IS LESS.
- FOR MORE ACCURATE FOUNDING MATERIALS, FOUNDING DEPTHS AND FURTHER RECOMMENDATIONS REFER TO SOIL REPORT.

CONCRETE PLACEMENT

CONCRETE SHALL NOT BE MIXED OR PLACED AT ANY TIME WHEN THE SHADE TEMPERATURE EXCEEDS 38°C OR IS LESS THAN 4°C. THE CONCRETE SHALL BE THOROUGHLY WORKED INTO ALL CORNERS, AROUND ALL EMBEDDED ITEMS INCLUDING REINFORCEMENT, AND BE WELL COMPACTED BY MEANS OF HIGH FREQUENCY MECHANICAL VIBRATORS AND TAMPING. ALL VIBRATORS SHALL BE OPERATED BY TRAINED OPERATORS AND STANDBY VIBRATORS SHALL BE AVAILABLE ON SITE FOR IMMEDIATE USE IN CASE OF BREAKDOWNS. VIBRATORS SHALL BE UNIFORMLY SPACED AT DISTANCES NOT EXCEEDING THE RADIUS AT WHICH VIBRATION IS VISIBLY EFFECTIVE, AND SHALL BE INSERTED INTO THE CONCRETE PROGRESSIVELY AS IT IS PLACED. NO PART OF THE CONCRETE SHALL BE OVER-VIBRATED, AND IN ANY CASE VIBRATIONS IN ANY REGION SHALL NOT EXCEED 20 SECONDS. IF RESTRICTED SPACE MAKES INTERNAL VIBRATION OF CONCRETE DIFFICULT, EXTERNAL FORM VIBRATORS MAY BE USED, ASSISTED BY HAMMERING AND TAPPING OF THE FORMWORK IF NECESSARY TO ENSURE COMPLETE FILLING OF THE FORMS.

CONCRETE CURING

AFTER PLACEMENT AND FINISHING OF CONCRETE, ALL EXPOSED SURFACES SHALL BE PROTECTED AGAINST PREMATURE DRYING. IN WINDY CONDITIONS, SUITABLE WIND BREAKS SHALL BE ERECTED TO SHIELD THE CONCRETE DURING PLACEMENT AND CURING. AS SOON AS THE SURFACES OF CONCRETE HAVE HARDENED SUFFICIENTLY TO AVOID MARKING OR OTHER DAMAGE, BUT IN NO CASE LESS THAN THREE HOURS AFTER COMPLETION OF TROWELLING, THE SURFACE SHALL BE CURED BY SPRAYING WITH CLEAN WATER AND COVERING WITH AN APPROVED IMPERVIOUS MATERIAL (EG 0.1MM THICK POLYETHYLENE SHEETING) FOR A MINIMUM OF SEVEN DAYS. TO PREVENT DISPLACEMENT BY WIND, ALL COVERINGS MUST BE WELL WEIGHTED, PARTICULARLY AROUND THE EDGES AND ALONG LAPPED JOINTS. DURING HOT WEATHER, SIDES OF FORMS SUPPORTING CONCRETE SHALL BE KEPT COOL BY SHADING AND/OR WETTING THROUGHOUT THE CURING PERIOD. IF FORMS ARE STRIPPED DURING THE CURING PERIOD, THEN THE EXPOSED CONCRETE SURFACES SHALL BE KEPT WET AND COVERED DURING THE REMAINDER OF THE CURING PERIOD. AT THE COMPLETION OF THE SEVEN DAY CURING PERIOD, THE CONCRETE SHALL NOT BE PERMITTED TO DRY RAPIDLY. IF NECESSARY, THE CONCRETE SHALL BE SPRINKLED WITH CLEAN WATER AT REGULAR INTERVALS TO RETARD THE RATE OF DRYING. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL CONCRETE IS CURED CORRECTLY AND TO TAKE ALL PRECAUTIONS NECESSARY AND ADVISABLE FOR ITS PROTECTION.

SLAB-BASE PREPARATION NOTES

- Remove top soil vegetation and humus from area under proposed slab.
- Remaining clean material is to receive a minimum of six passes of a medium weight vibratory roller.
- Any soft spots are to be dug out and replaced with fine crushed rock.
- Filling under slab if required shall be crushed rock and/or salomander laid down in 150mm maximum depth layers and compacted to 95% of the standard value. Compaction testing must be carried out to certify degree of compaction specified.
- To the underside of slab place 50mm sand and consolidate sufficiently to support bar chairs.
- Lay 0.2mm polythene sheets below the whole of slab and lap joints 450mm minimum. cut around pipes and seal with tape. Repair any holes before pouring.
- Provide 300mm minimum lap to SL82 mesh. provide 500mm minimum lap to 11mm.

BLOCKWORK NOTES

- Blockwork to be in accordance with AS 3700.
- Blocks used on this project shall have a characteristic unconfined compressive strength $f_{cu} = 12$ MPa.
- Mortar in blockwork shall be:
 - Generally: 1 cement : 1 lime : 6 sand
 - Reinforced blockwork: 1 cement : 1/4 lime : 3 sand
- Blockwork must not be built off propped slabs.
- All blockwork to be tied to structural members at every 400mm centres max. Unless otherwise noted, use 6mm dia. minimum ties.
- Wall ties shall comply with AS 2699 and AS 2975 and spaced at 600mm max. centres horizontally and 400mm max. centres vertically.
- Blockwork control joints to be specified by architect or at 6.0m centres max.
- All non-load bearing blockwork to be kept clear of underside of slab and beams by 20mm U/L/O.
- Provide 2 layers of malthoid where concrete bears on blockwork.
- Lintels and beams to be supported on solid blocks or hollow blocks filled with mortar.

NOTES

GENERAL

- These drawings shall be read in conjunction with all architectural and other consultants' drawings and specifications and with such other written instructions as may be issued during the course of the contract. All discrepancies shall be referred to the Architect for decision before proceeding with the work.
- All dimensions relevant to setting out and off site work shall be verified by the Contractor before construction and fabrication is commenced. The Engineer's drawings shall not be scaled.
- During construction the Contractor shall be responsible for maintaining the structure in a stable condition and ensuring no part shall be overstressed under construction activities.
- Workmanship and materials shall be in accordance with the relevant current S.A.A. codes including all amendments, and the local Statutory Authorities, except where varied by the contract documents.
- The approval of a substitution shall be sought from the Engineer, but it is not an authorisation for an extra. An extra involved must be taken up with the Architect before the work commences.
- All dimensions are in millimetres unless stated otherwise. All levels are expressed in millimetres.
- The structural work shown on these drawings has been designed for the following live loads:-

AREA	LIVE LOAD kN/sq.m
ROOF	0.25
GARAGE	3.00
FLOOR	1.50

- All props and formwork for beams and slabs shall be removed before construction of any masonry walls or partitions on the floor.
- All non loadbearing walls shall be kept clear of the underside of slabs and beams by 20 mm unless otherwise shown.
- Abbreviations used T-top, B-bottom, E.W.-each way, pl-plate, c.f.w.-continuous fillet weld, u.n.a.-unless noted otherwise.

FOOTINGS

Footings are to be founded in original undisturbed ground having a safe bearing capacity as noted below. Before any concrete is placed the safe bearing capacity shall be verified by a qualified Engineer.

Pad Footings	Strip Footings	Ribs	Bored Piers
100 kPa	100 kPa	100 kPa	100 kPa
MIN.	MIN.	MIN.	MIN.

CONCRETE

- All workmanship and materials shall be in accordance with AS 3600.
- Minimum cover (mm) to all reinforcement unless otherwise shown shall be as follows:-

ELEMENT	FORMED & NOT EXPOSED TO WEATHER	FORMED & EXPOSED TO GROUND, WEATHER OR WATER	NOT FORMED, CAST AGAINST GROUND ETC.
Columns & Pedestals	40	50	75
Beams	40	50	65
Footings	-	65	75
Slabs	20	-	65
Walls	25	30	65

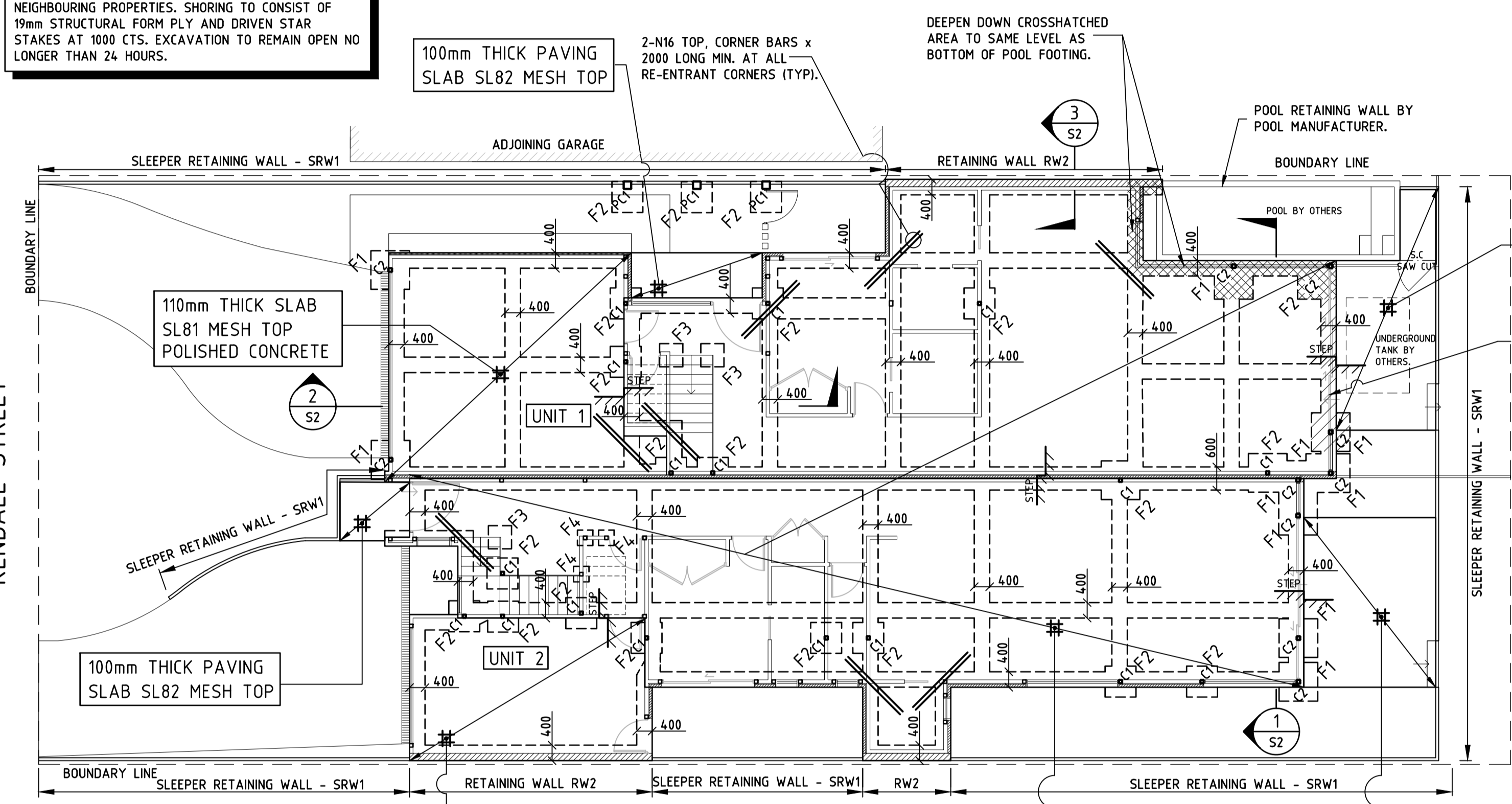
- Sizes for concrete elements do not include thickness of applied finishes.
- Beam depths are written first and do not include slab thickness.
- No holes, chases or embedment of pipes other than those shown on the structural drawings shall be made in concrete members without prior approval of the Engineer.
- Construction joints shall be properly formed and used only where shown or specifically approved by the Engineer.
- Reinforcement is represented diagrammatically and not necessarily shown or specifically approved by the Engineer.
- Splices in reinforcement shall be made only in the positions shown or as otherwise approved by the Engineer.
- Welding of reinforcement shall not be permitted without the approval of the Engineer.
- All reinforcement shall be supported in its correct position during concreting by approved bar chairs, spacers or support bars.
- Reinforcement symbols-
S Structural grade deformed bar to AS 1302
N500 grade deformed bars to AS 1302
R Structural grade plain round bar to AS 1302
SL Hard drawn steel wire reinforcing fabric to AS 1304
The number following the bar symbol is the nominal bar diameter in millimetres.
- Camber formwork to beams and slabs shall be set to a pre-determined level (allowing for immediate deflection of supporting structure and formwork settlement) to give zero negative camber immediately after concrete placement. Additional positive cambers shall be as noted on the drawings.
- Stripping of formwork to be in accordance with Table 19.6.2.5 of AS 3600.
- Formwork shall be designed and constructed in accordance with AS 1509.
- Concrete to be kept free of supporting brickwork by two layers of malthoid.
- Concrete testing to be in accordance with AS 3600.
- Concrete components and quality shall be as follows.

ELEMENT	f_c MPa	SLUMP (mm)	MAX AGGREGATE SIZE (mm)
FOOTINGS	20	80	20
BLINDING	15	80	20
SLABS	25	80	20

STRUCTURAL STEELWORK

- All workmanship and materials shall be in accordance with AS 1250 and/or AS 4100.
- Welding shall be performed by an experienced operator in accordance with AS 1554.
- High Strength Friction Grip bolting shall be in accordance with AS 1551.
- The Contractor shall provide and leave in place until permanent bracing elements are constructed such temporary bracing as is necessary to stabilise the structure during erection.
- Concrete encased steelwork, shall be wrapped with F41 brick, unless otherwise shown.
- The ends of all tubular members are to be sealed with nominal thickness plates and continuous fillet weld unless otherwise shown.
- Camber to be as noted on the drawings.
- Before fabrication is commenced the Contractor shall submit copies of the shop drawings to the Architect for review. Review does not include checking of dimensions.
- Unless otherwise specified all steelwork shall be painted one shop coat of Red Oxide Zinc Chromate primer. Members encased in concrete, fire sprayed or friction grip bolted connections must not be painted.
- Except where otherwise shown the welds to be 6mm continuous fillet and E41 xx Electrodes.
- Steelwork below ground to have 75 concrete cover.
- Builder to be responsible for field welding of masonry ties to steel sections.

NOTE: BUILDER TO TEMPORARILY SHORE FOOTING EXCAVATIONS ALONG BOUNDARY LINE TO MAINTAIN ALL NEIGHBOURING PROPERTIES. SHORING TO CONSIST OF 19mm STRUCTURAL FORM PLY AND DRIVEN STAR STAKES AT 1000 CTS. EXCAVATION TO REMAIN OPEN NO LONGER THAN 24 HOURS.



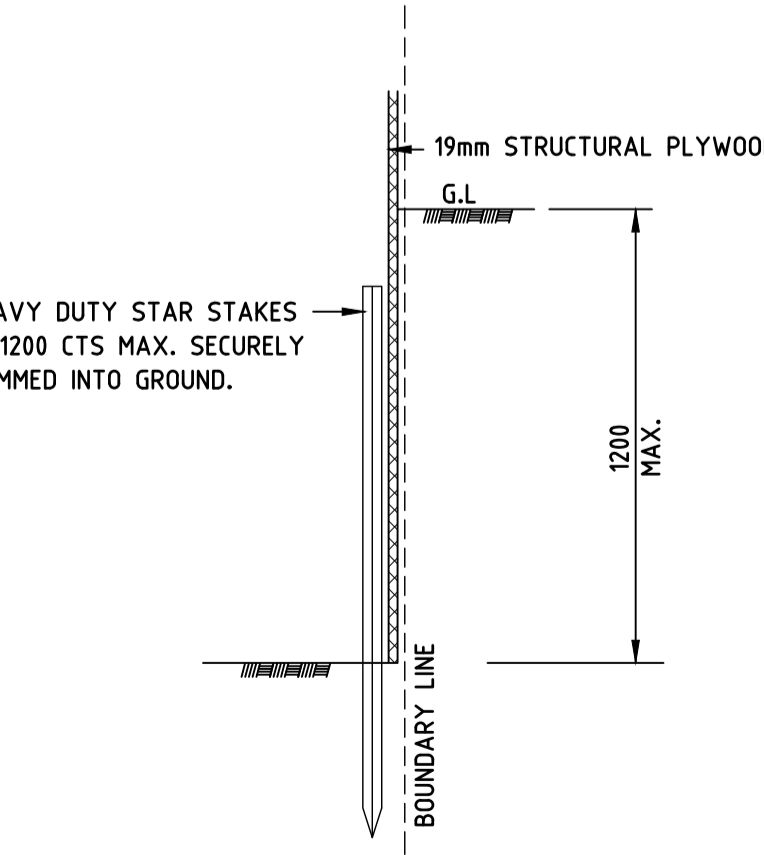
150mm THICK PAVING SLAB SL82 MESH TOP AND BOTTOM

DEEPEN DOWN HATCHED AREA TO THE SAME LEVEL AS THE BOTTOM OF UNDERGROUND TANK.

PAD FOOTINGS SCHEDULE

MARK	WIDTH	LENGTH	DEPTH (min)	REINFORCEMENTS
F1	1000	1000	800	SL81 MESH BOTTOM
F2	800	800	800	SL81 MESH BOTTOM
F3	600	600	800	SL81 MESH BOTTOM
F4	400	400	800	-

NOTE: BUILDER TO ENSURE FILL UNDER SLAB ON GROUND DOES NOT EXCEED 400mm. THIS OFFICE TO BE NOTIFIED IF FOUND OTHERWISE SO THAT A SUSPENDED SLAB CAN BE DESIGNED.



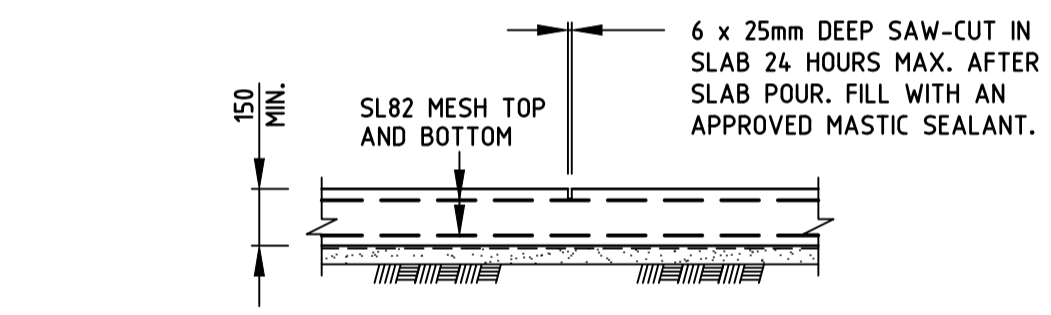
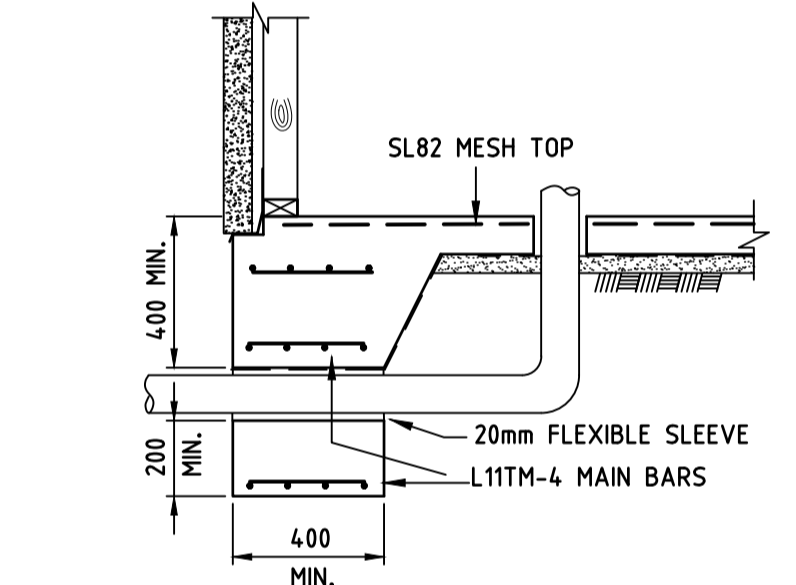
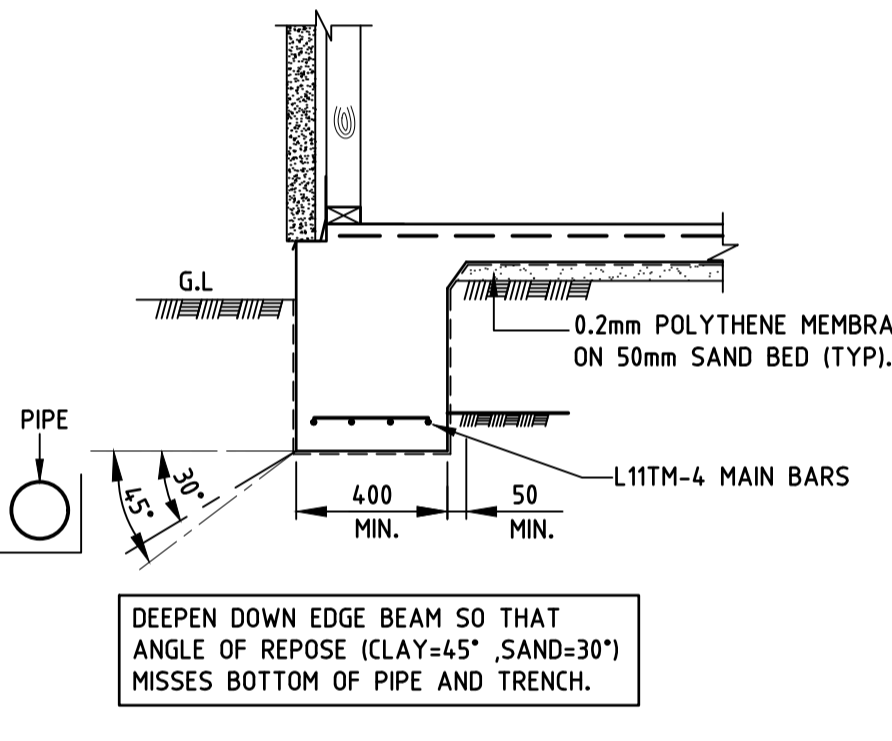
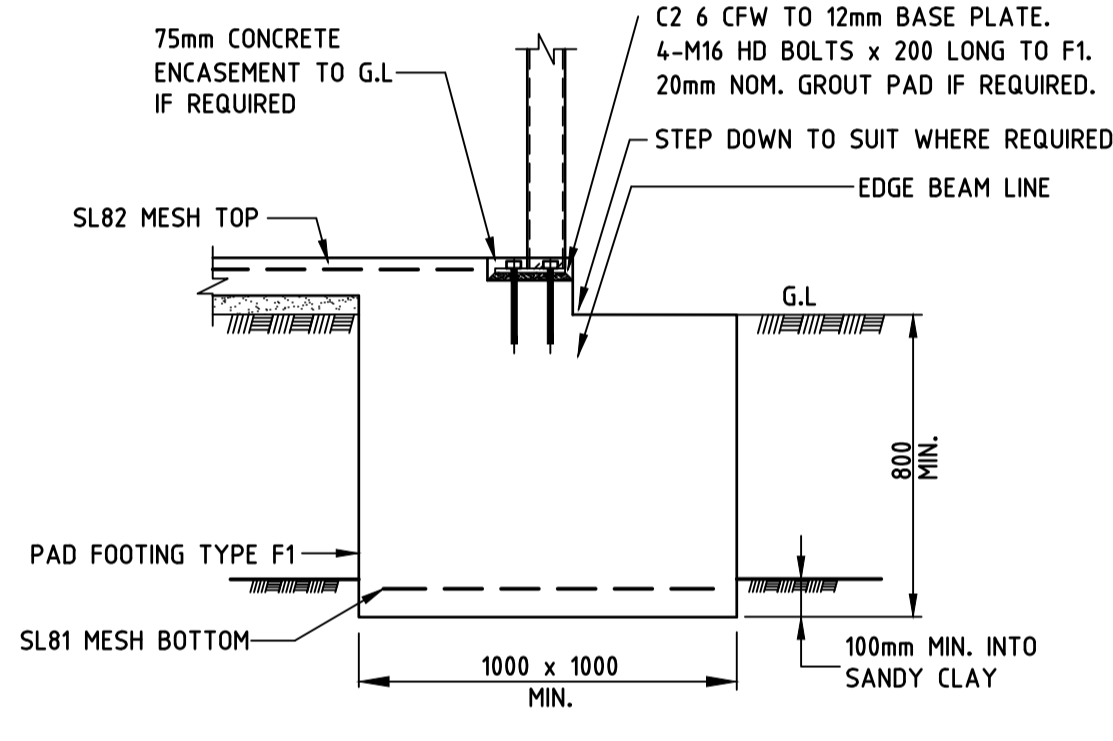
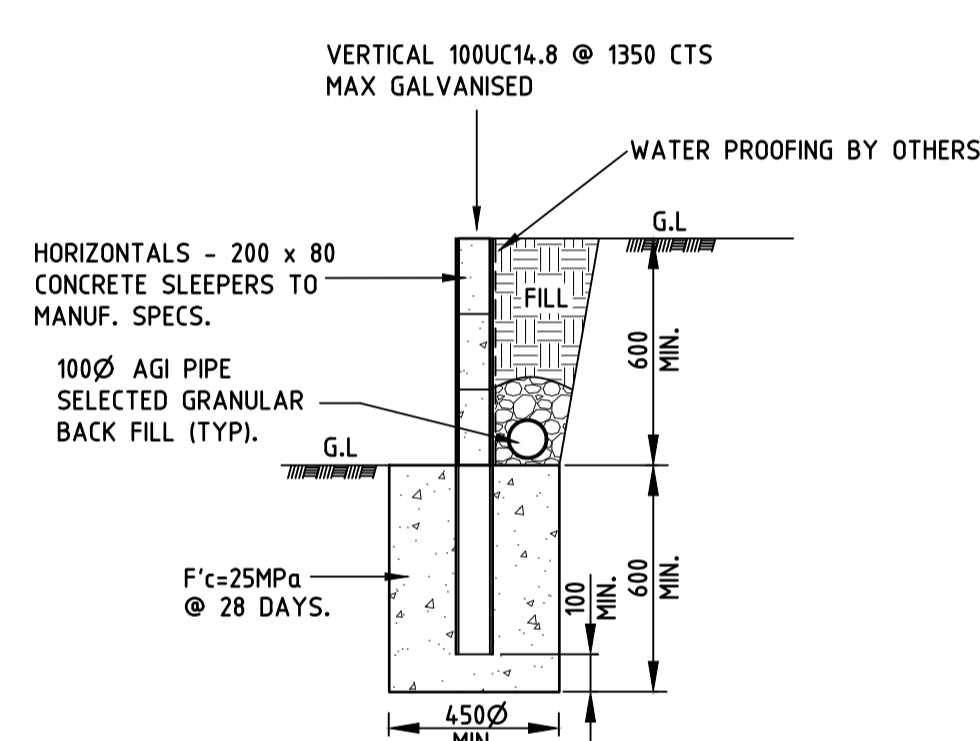
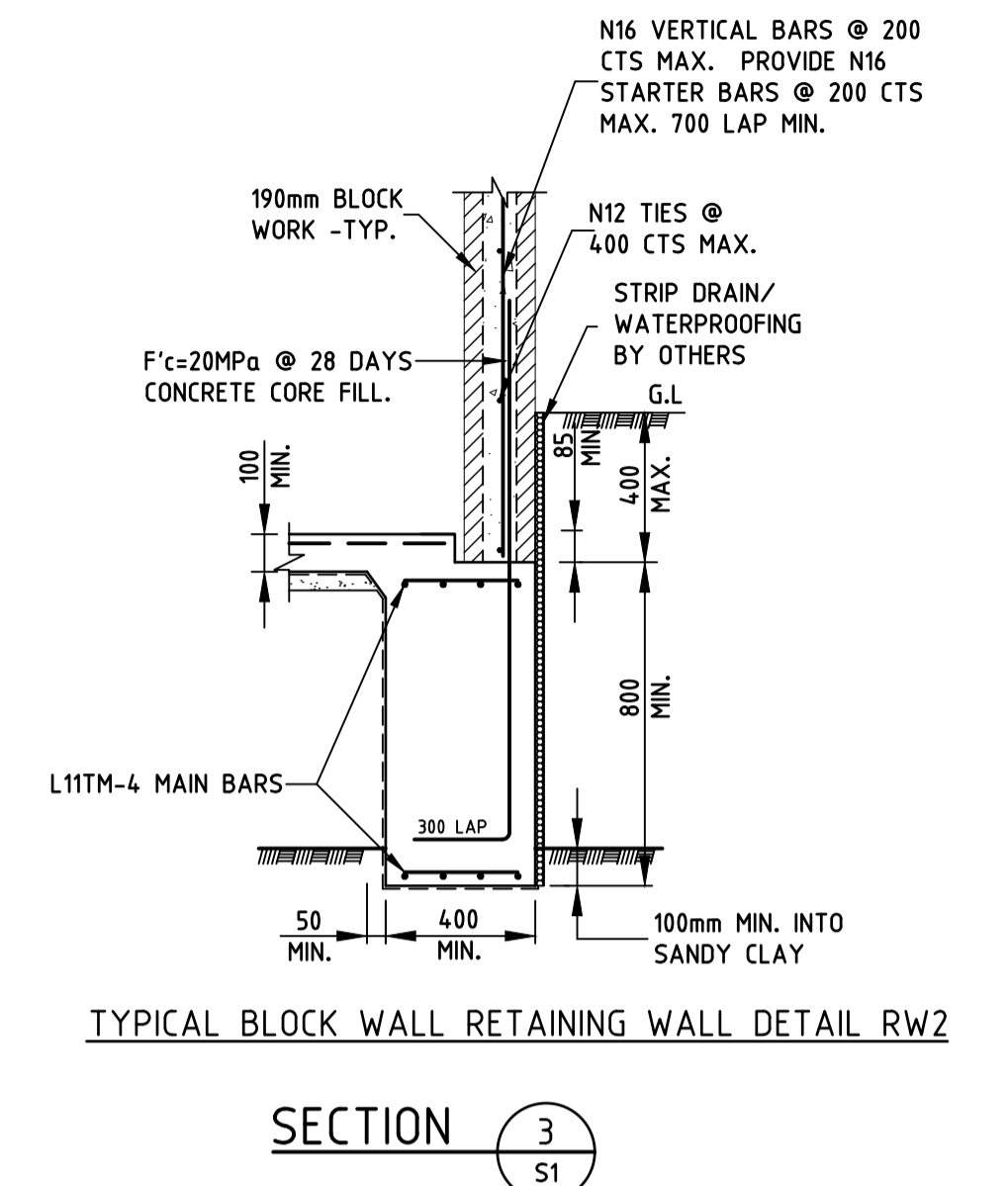
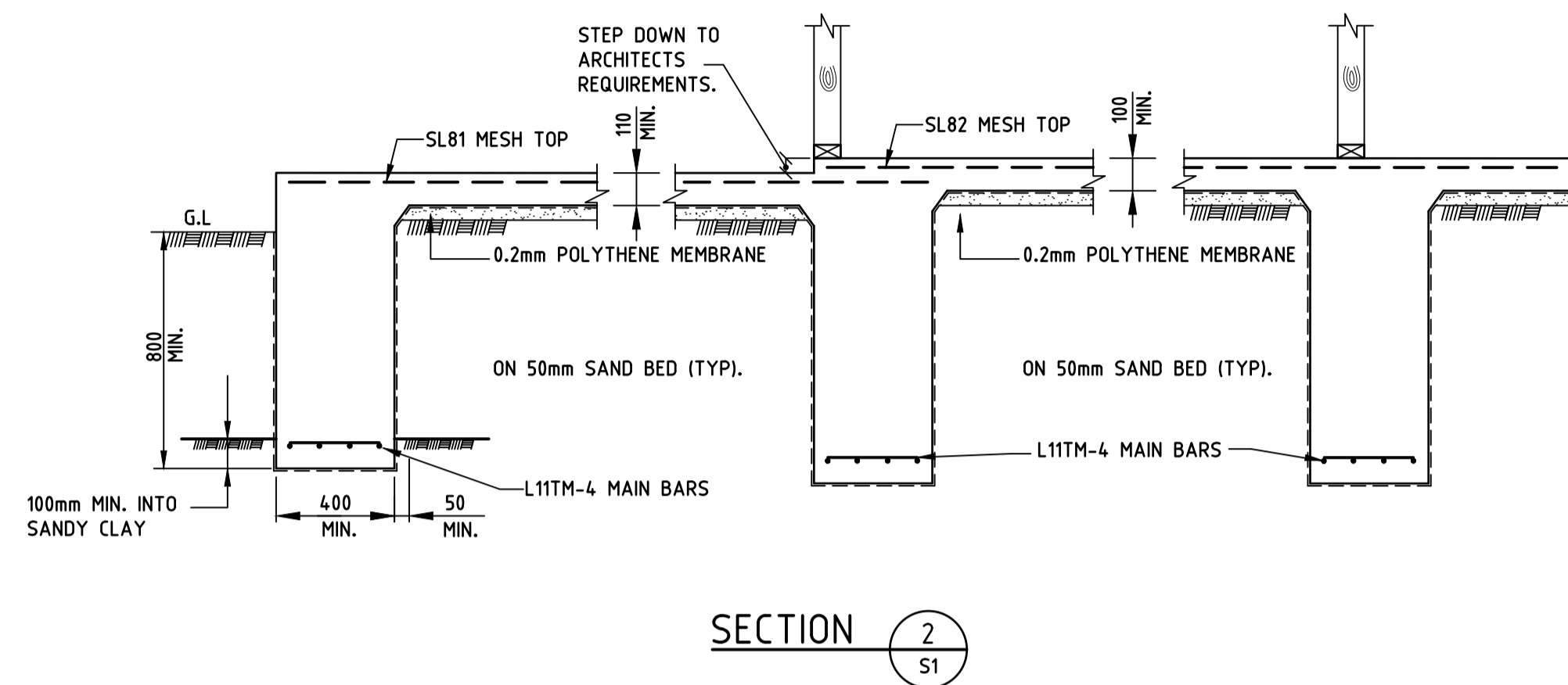
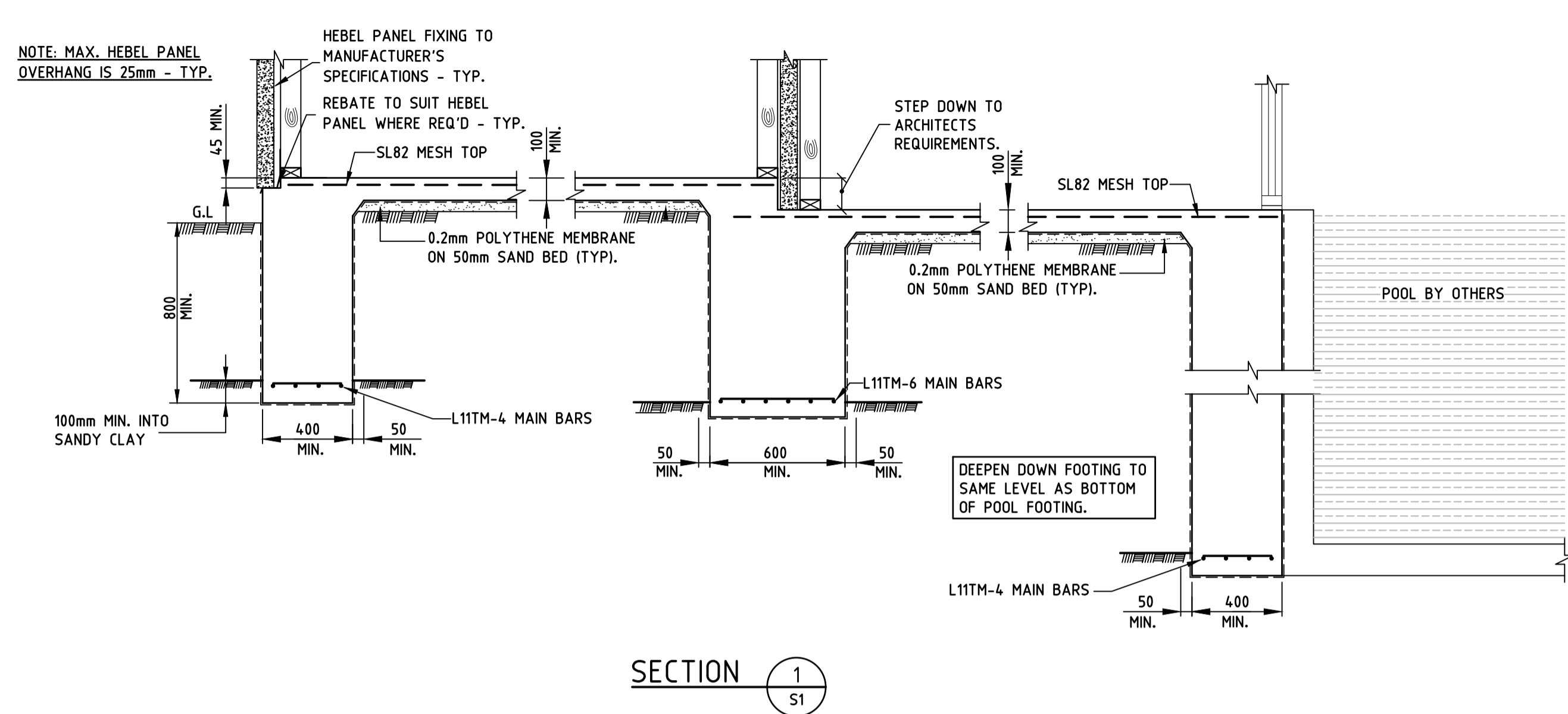
TEMPORARY SHORING DETAIL

REVISION	DESCRIPTION	DRAWN	DATE
C1	ISSUED FOR CONSTRUCTION	ND	07/02/24
P2	PRELIMINARY 2	ND	02/02/24
P1	PRELIMINARY 1	ND	06/12/23

DSL CONSULTING PTY LTD
 Consulting Structural Engineers
 A.B.N: 85 611 336 291
 659 High Street
 Kew East, VIC 3102
 T: 03 9859 1611
 E: structures@dsiconsulting.com.au
 W: www.dsiconsulting.com.au

CLIENT:	MR AND MRS VARGHESE
ARCHITECT:	ARKI DESIGN STUDIO TEL: 03 9989 5118 EMAIL: PETER@ARKIDESIGN.COM.AU

PROJECT:	PROPOSED DUAL OCCUPANCY AT 32 KENDALL STREET HAMPTON		
SCALE AT:	DATE:	DESIGNED:	DRAWN:
1:100, 1:20	NOV. 2023	DAN LAU	ND
DRAWING NO:	4363-23	S1 of 4	REVISION: C1



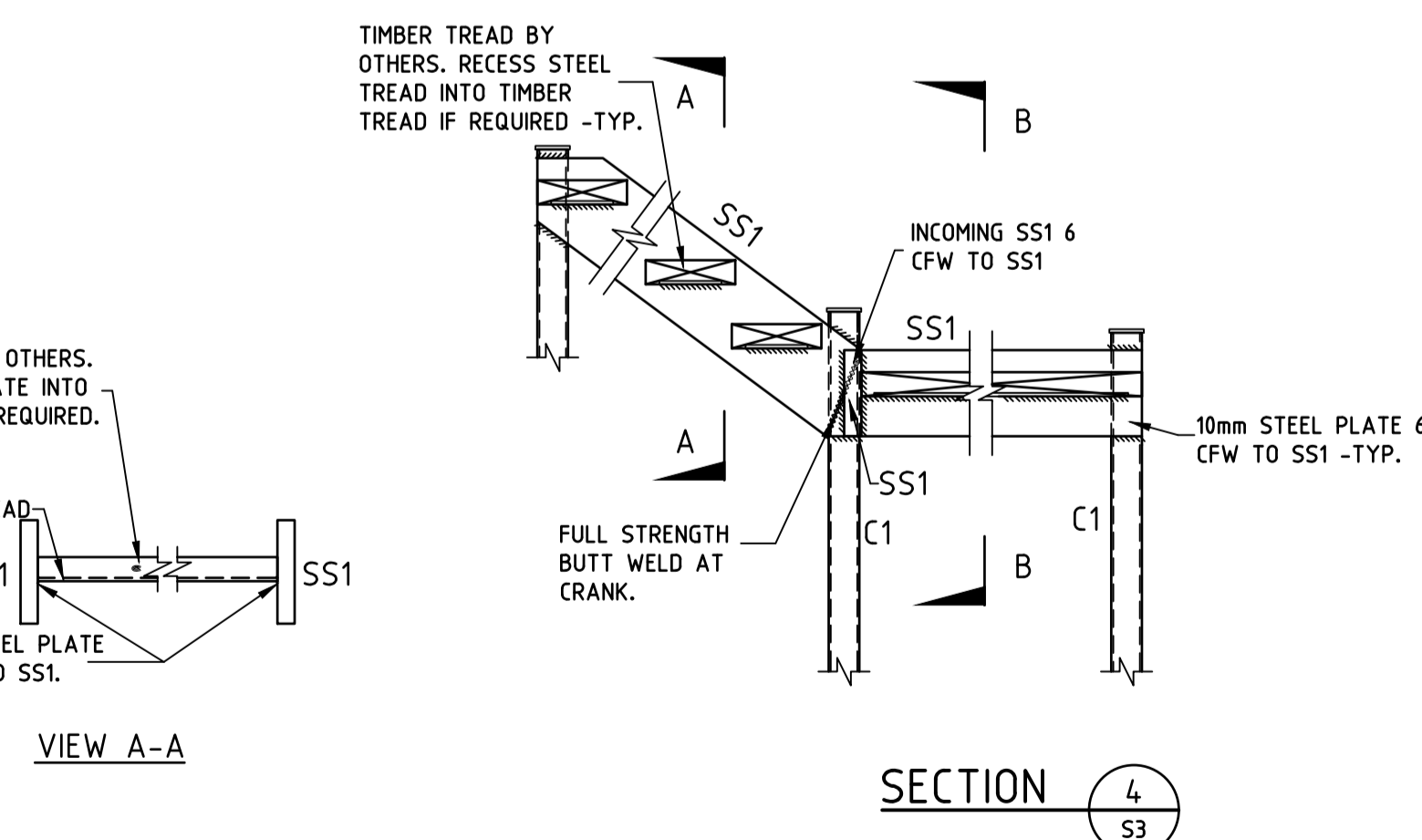
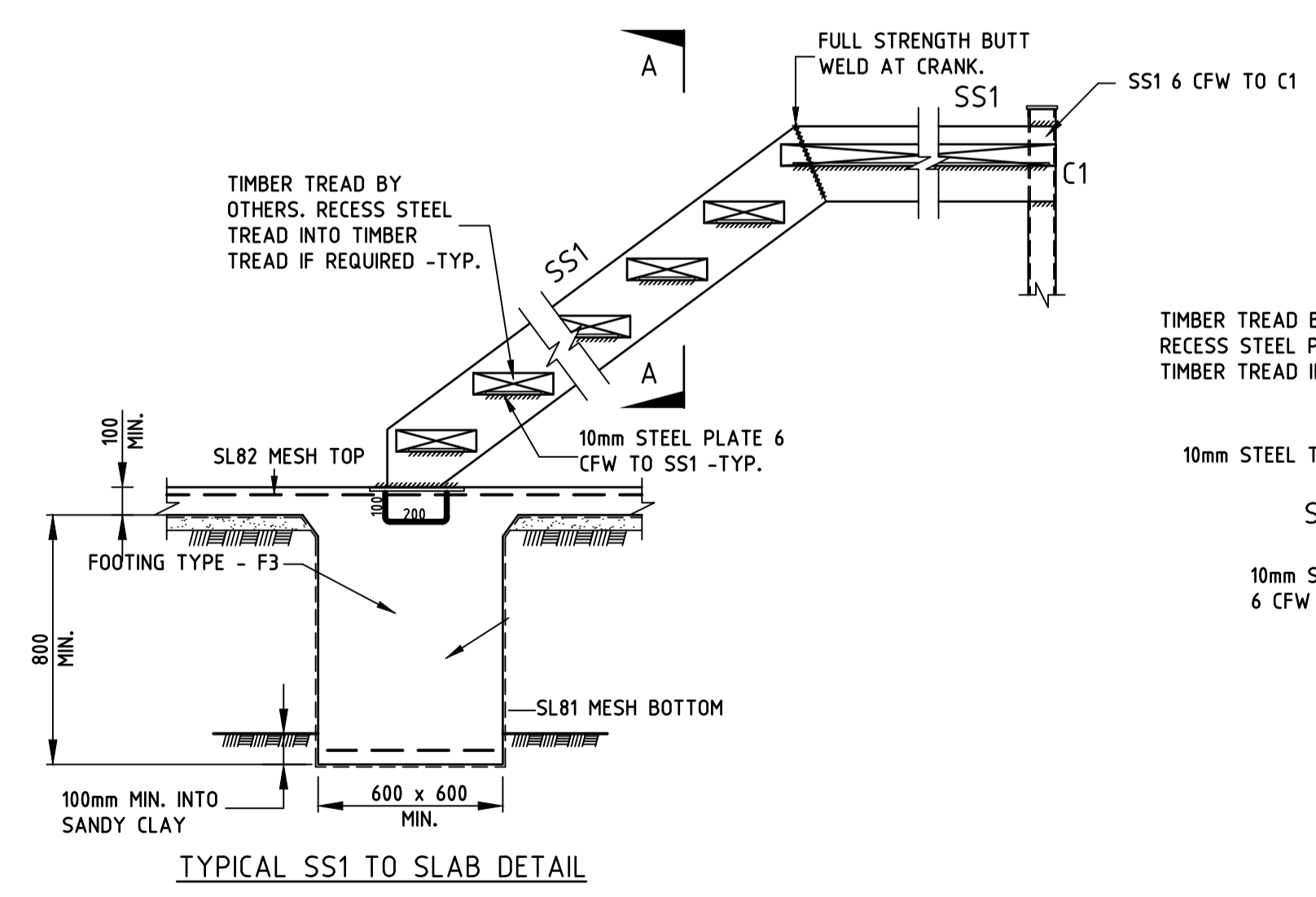
TYPICAL RETAINING WALL DETAIL - SRW1

FOOTING DETAIL - C2 TO F1
SIMILAR FOR C1 TO F2
SIMILAR FOR PC1 TO F2

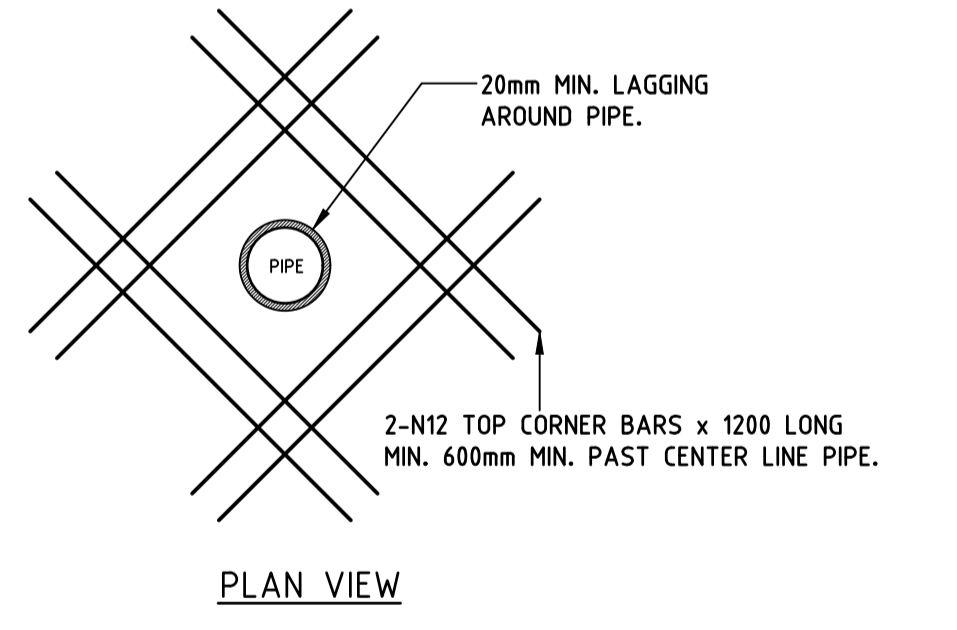
TYPICAL ANGLE OF REPOSE DETAIL
(TYPICAL WHERE APPLICABLE)

PIPE THROUGH EDGE BEAM DETAIL
(TYPICAL WHERE APPLICABLE)

TYPICAL SAW-CUT DETAIL - SC



UNIT 2 STAIRS



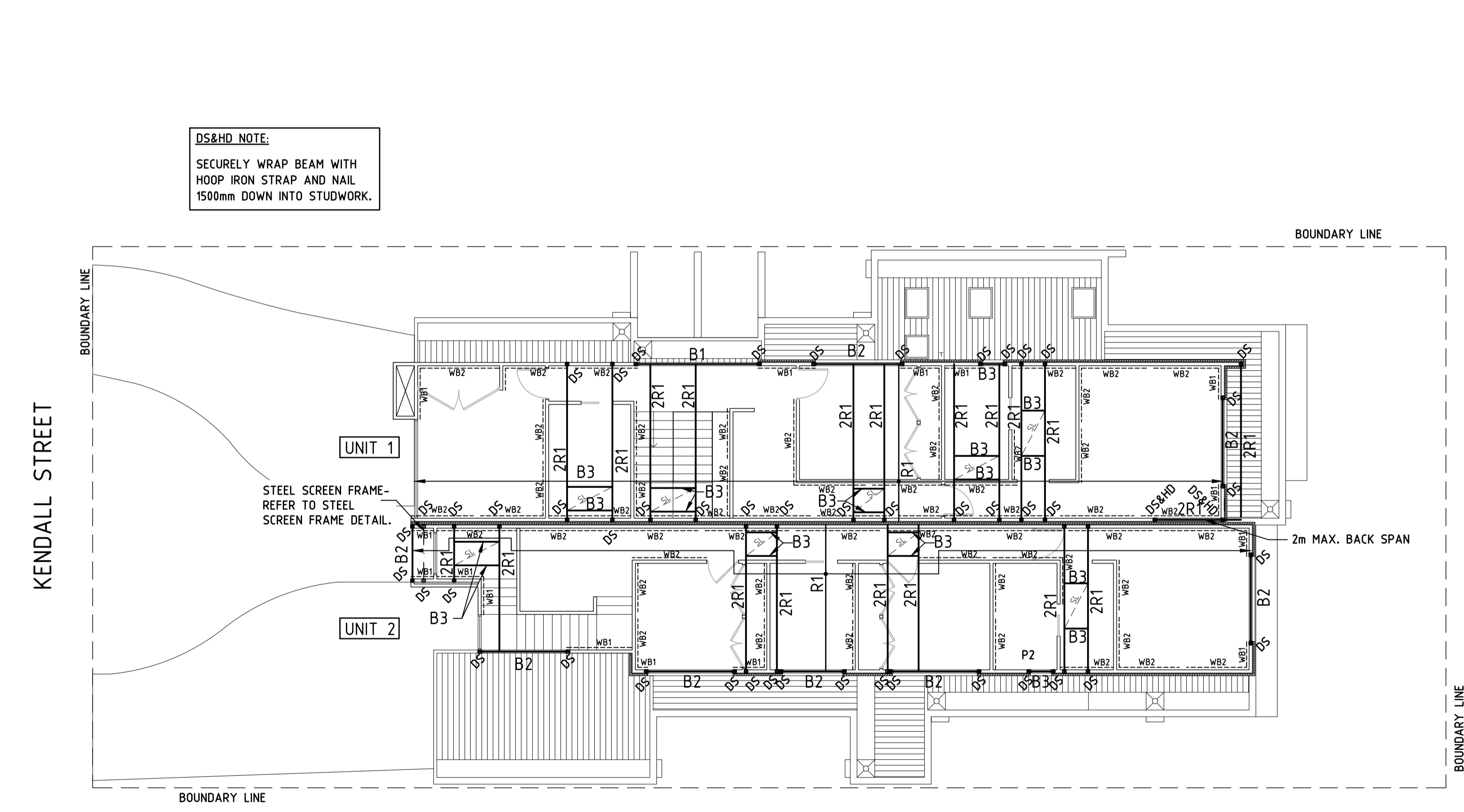
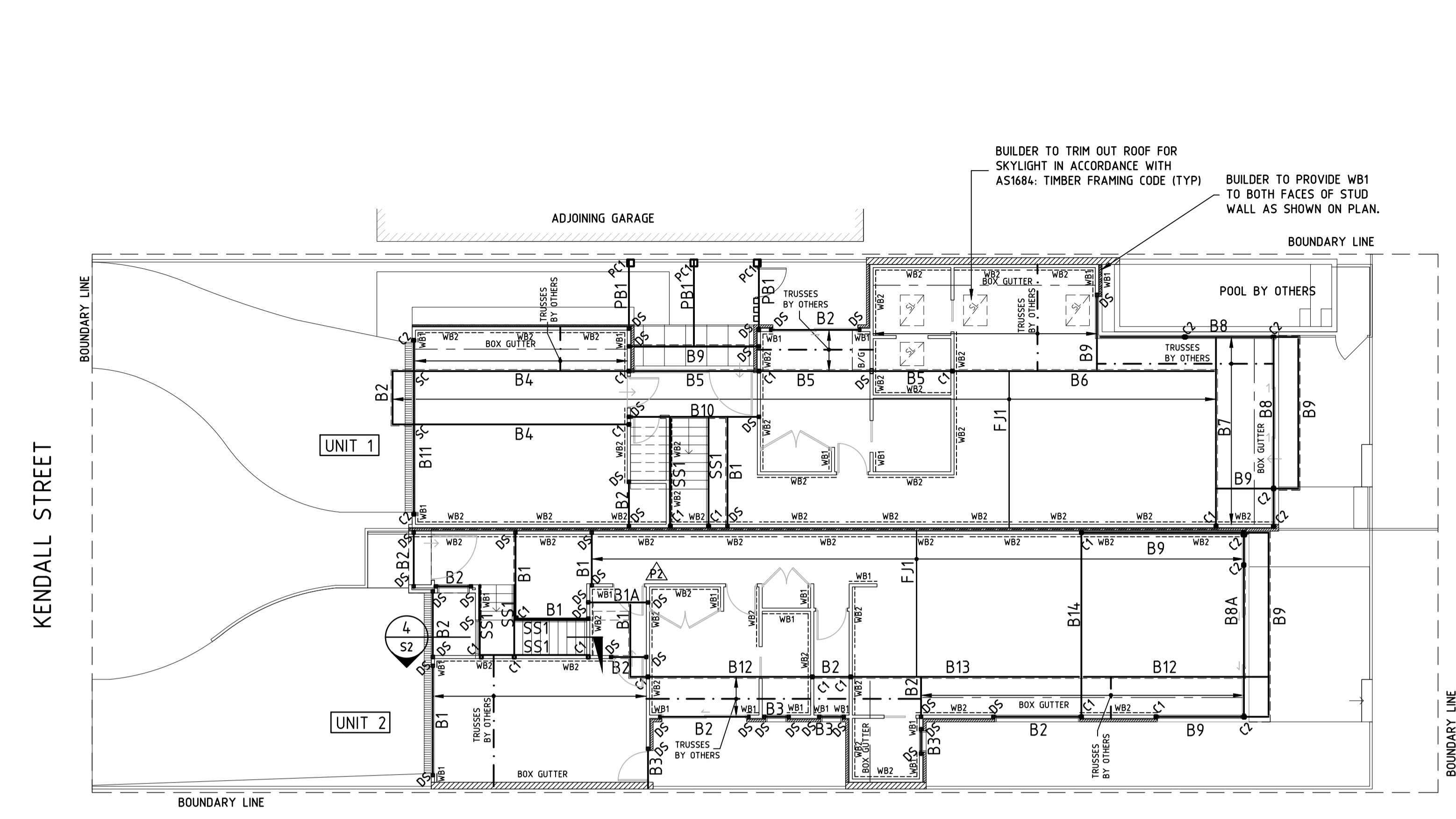
TYPICAL REINFORCEMENT AROUND PIPE DETAIL

UNIT 1 STAIRS

STAIR DETAILS

C1	ISSUED FOR CONSTRUCTION	ND	07/02/24	CLIENT:	MR AND MRS VARGHESE
P2	PRELIMINARY 2	ND	02/02/24	ARCHITECT:	ARKI DESIGN STUDIO TEL: 03 9989 5118 EMAIL: PETER@ARKIDESIGN.COM.AU
P1	PRELIMINARY 1	ND	06/12/23	PROJECT:	PROPOSED DUAL OCCUPANCY AT 32 KENDALL STREET HAMPTON
REVISION	DESCRIPTION	DRAWN	DATE	SCALE AT A1:	1:20
				DATE:	NOV. 2023
				DESIGNED:	DAN LAU
				DRAWN:	ND
				DRAWING NO:	4363-23
				REVISION:	S2 of 4
					C1

DSL CONSULTING PTY LTD
Consulting Structural Engineers
A.B.N: 85 611 336 291
659 High Street
Kew East, VIC 3102
T: 03 9859 1611
E: structures@dslconsulting.com.au
W: www.dslconsulting.com.au



UPPER ROOF FRAMING PLAN
(ROOF TRUSSES TO MANUFACTURER'S SPEC. TYP.)

NOTE:
ALL WALL STABILITY, ROOF STABILITY AND SAFETY DURING CONSTRUCTION ARE THE BUILDER'S RESPONSIBILITY.

STUD SCHEDULE:

EXTERNAL STUDS:	DESCRIPTION
STUD HEIGHT UP TO 3200	90 x 45 MGP10 @ 450 CTS.
3200 TO 4500	2/90 x 45 MGP10 @ 450 CTS.
4500 TO 5300	2/90 x 45 F17 KDHW or F17 L.V.L @ 450 CTS.

INTERNAL STUDS:	DESCRIPTION
STUD HEIGHT UP TO 3600	90 x 45 MGP10 @ 450 CTS.
3600 TO 4500	2/90 x 45 MGP10 @ 450 CTS.
4500 TO 5300	2/90 x 45 F17 KDHW or F17 L.V.L @ 450 CTS.

NOGGINGS @ 1350 MAX. CTS.

TEMPORARY BRACING

TEMPORARY BRACING (SUCH AS SUPPORTS, GUYS, BRACES, FALSEWORK, CRIBBING, ETC) IS NOT PROVIDED BY DSL CONSULTING AND MUST BE USED THROUGHOUT THE ENTIRE ERECTION PROCESS FOR ALL BUILDINGS TO SECURE THE STEEL FRAMING AGAINST LOADS RESULTING FROM THE WIND, SNOW OR ANTICIPATED ERECTION LOADS.

A. THE PURPOSE OF TEMPORARY BRACING IS TO ADEQUATELY TRANSFER LOADS TO THE GROUND FROM THE FRAME OR OTHER MEMBERS.

B. DURING ERECTION, THE BUILDING STRUCTURE IS NOT STABLE WITHOUT TEMPORARY BRACING.

1. THE EFFECT OF WIND LOADS ON THE COMPLETED STRUCTURE MAYBE GREATER THAN THOSE ON A COMPLETE STRUCTURE.

2. ANCHOR BOLTS AND FOOTINGS ARE NOT ADEQUATE FOR TEMPORARY BRACING.

C. IT IS THE RESPONSIBILITY OF THE ERECTOR/ BUILDER TO DETERMINE THE AMOUNT AND SIZE OF TEMPORARY BRACING AND TO FURNISH AND INSTALL ALL TEMPORARY BRACING.

NOTE ON VARIATIONS:

THE APPROVAL BY THIS OFFICE OF A SUBSTITUTE MATERIAL, WORK PRACTICE, VARIATION OR THE LIKE IS NOT AN AUTHORISATION FOR ITS USE OR A CONTRACT VARIATION. ANY SAID VARIATIONS MUST BE ACCEPTED BY ALL PARTIES TO THE AGREEMENT AND WHERE APPLICABLE THE RELEVANT BUILDING SURVEYOR PRIOR TO IMPLEMENTING THE SAID VARIATION.

NOTE: ALL TIMBER EXPOSED TO THE WEATHER TO BE "H3" TREATED OR SIMILAR (U.N.O.).

NOTE: ALL STEELWORK (INCL. CLEATS, BOLTS, WASHERS ETC.) EXPOSED TO THE WEATHER SHALL BE HOT DIP GALVANISED.

NOTE: ALL F17 KDHW MEMBERS TO BE SOLID TIMBERS (NOT SPLICED & PLATED)

PREFABRICATED TIMBER TRUSSES

The prefabricated trusses shall be designed and manufactured by a fabricator approved by the Consulting Engineer.

The Building Contractor shall be responsible for this design, the cost of which shall be included in his total tender price.

Amendments to the design to meet any requirements contained in the drawings, specifications or AS 1720 do not entitle the Contractor to an extra.

The truss engineering drawings (including details of truss to truss connections, any additional stiffeners, packers and fasteners), truss computations and a drawing showing bracing and assumed truss restraints shall be submitted to the Consulting Structural Engineer for record purposes prior to commencement of truss fabrication. These drawings shall be checked and signed as being correct by the Truss Design Engineer.

The erection of the trusses shall be preferably by the fabricator. Where this is not possible, the Contractor shall obtain and familiarise himself with all necessary and complete information on the correct storage, handling and erection (including truss to truss and truss to support connections, truss chord and web stiffeners, chord and web restraints and bracing requirements). The Contractor shall allow and arrange for at least one site inspection of the trusses by the Truss Design Engineer after completion of erection and bracing and submit a report to the Consulting Engineer. The report must state that the fabrication, erection and tie-down details of all timber trusses have been inspected and that they comply with the design.

The Engineer's report is to be sent to the Consulting Structural Engineer.

The truss design must comply with the following requirements:

- The design shall be in accordance with AS 1720 Timber Engineering code, AS 1684 Light Timber Framing code where applicable.
- All truss timbers to be K.D. Hardwood (Grade F17) or seasoned structural Radiata Pine (Grade F9) except for exposed roof trusses which are to be fine sawn Oregon (Grade F7) or K.D. Hardwood (Grade F17) depending on member sizes and architectural requirements.
- Truss supports and spans to be as shown on structural drawings.
- Concealed trusses shall have a maximum spacing of 900mm (U.N.O.) for Metal Roof and 450mm (U.N.O.) for Terra Cotta.
- Roof slopes are as shown on architectural drawings.
- Design data are as follows:-
 - Roof material: Metal Sheet/ Terra Cotta
 - Ceiling material: 10mm plasterboard
 - Services: Trusses to be designed to support all service equipment, pipes and ductwork shown on the relevant service drawings. The entire roof area shall be designed for a minimum blanket load for services of 0.10 kN/sq.m.
 - Deflection: Maximum long term dead load deflection between truss supports to be the lesser of span/300 or 15mm measured from the initial preambered position to the final loaded position. Provide preamber to truss equal to the long term dead load deflection. Maximum load load deflection between truss supports to be the lesser of span/300 or 15mm.
 - Wind: Terrain category = 3.0
Basic wind velocity = 45 m/s
Minimum internal pressure coefficient = +0.3, 0
 - Maintenance: Where trusses are concealed a point load of 1.3 kN for maintenance shall be allowed for on the bottom chord of each truss.
 - Occasional Load: Exposed trusses shall be designed for clause 3.8.3 of AS 1170 Part 1.
 - Live Load: Roof live load to be in accordance with AS 1170 Part 1.
- All temporary bracing necessary to erect and locate trusses in position until permanent bracing is installed is to be provided by the truss fabricator and installed to the truss designer's specification. Additional permanent bracing to that shown on the structural drawings is required to provide stability to the roof trusses. It is the Truss Fabricator's responsibility to design and provide this additional bracing.
- Roof bracing shown on the structural drawings consists of 10mm thick galvanized steel angle braces with 20mm legs bent over wall top plate, truss, beam or rafter at ends and nailed with 5 No. 30 x 3.15mm flat head galvanized nails. Bracing is to be hammered flat over each truss top chord or rafter and fixed with 2 No. 30 x 3.15mm flat head galvanized nails. Contractor is to ensure bracing straps are fast when roof framing has been completed.
- All truss to truss and truss to wall connections are to be the responsibility of the Truss Designer. Allowance of wind uplift forces to be transferred from truss to truss shall be made in the connection design.
- The member sizes and truss geometry (ie web layout) for concealed trusses shall allow for mechanical ducting passing through the trusses. The Contractor and the Truss Designer are to co-ordinate the truss profiles with the mechanical and structural drawings to accommodate all ductwork.

ADDITIONAL TRUSS NOTES

- Truss design by others.
- Prior to commencing truss fabrication Builder must submit truss design, layout and details to Consulting Engineer so that design assumptions can be verified.
- All girder trusses must be supported off 2 No. 90 x 45 MGP12 pine studs (U.N.O.) down to ground level.
- All ends of beams supporting girder trusses must bear on 2 No. 90 x 45 MGP12 pine studs - Typical (U.N.O.)

ROOF ANCHORING NOTES

- Ends of all roof beams are to be restrained against wind uplift with 20 x 10mm hoop iron strap wrapped tightly over beam and connected to 1 metre minimum studwork via 3 No. 3.75mm x 38 long flat head nails. For brick wall, hoop iron strap to be set into mortar line at 1 metre minimum down.
- Ends of all rafters are to be restrained against wind uplift with 20 x 10mm hoop iron strap wrapped tightly around each rafter and connected to studs/beam via 3 No. 3.15mm flat head nails x 38 long. At studs, hoop iron strap to be connected to 1 metre minimum of studwork. For brick wall, hoop iron strap to be set into mortar line at 1 metre minimum down.

TIMBER NOTES

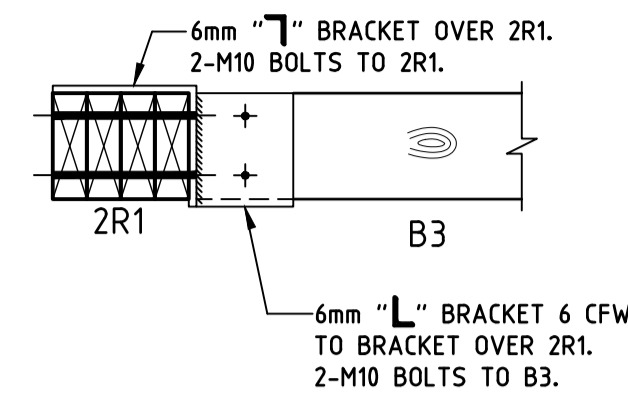
- All timber and workmanship to be in accordance with AS 1684 and AS 1720.
- All main roof and floor beams to be supported off 2/90 x 45 MGP12 studs down to ground level (Not necessarily shown on plan).
- Roof and wall framing to be in accordance with AS 1684.
- Provide solid nogginns at each end of every joint and at 1800 cts (max).
- Trimmer roof beams to be 190 x 45 F17 KDHW seasoned pine (U.N.O.) up to 2400mm opening.
- Under all walls parallel to joists use double floor joists, (U.N.O.)
- Studs which are adjacent to vertical steel members shall be fixed to the steel members with 3.7mm diameter x 75mm long powder actuated round shank drive pins at 100mm (maximum) from ends and at 600mm centres in between.

STEEL ANGLE LINTELS (U.N.O.)

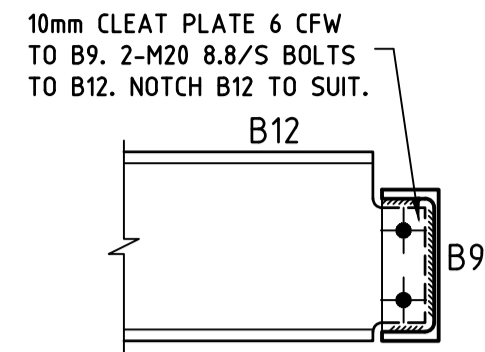
- Openings up to 1800mm - use 100 x 100 x 8 L's
Openings from 1800mm to 2700mm - use 150 x 100 x 10 L's (150 leg vertical).
- All lintels to have 230mm end bearing (U.N.O.)
- External lintels to be hot dipped galvanized.

MEMBER SCHEDULE	
MARK	SIZE
R1	140 x 45 F17 KDHW or F17 L.V.L @ 300 CTS. MAX.
ZR1	4/140 x 45 F17 KDHW or F17 L.V.L - SCREW LAMINATED
B1	2/240 x 45 F17 KDHW or F17 L.V.L - NAIL LAMINATED
B1A	2/190 x 45 F17 KDHW or F17 L.V.L - NAIL LAMINATED
B2	190 x 45 F17 KDHW or F17 L.V.L
B3	140 x 45 F17 KDHW or F17 L.V.L
B4	300 x 90 PFC
B5	2/300 x 45 F17 KDHW or F17 L.V.L - NAIL LAMINATED
B6	310UC118
B7	310UB46
B8	250 x 90 PFC
B8A	300 x 90 PFC
B9	200 x 75 PFC
B10	200 x 75 PFC
B11	380 x 100 PFC
B12	250 x 90 PFC
B13	380 x 100 PFC OR 360UB51 OR 310UC158
B14	310UC158
SS1	250 x 50mm MIN. STEEL PLATE - STAIR STRINGER
PB1	200 x 200 x 6.0 SHS
PC1	200 x 200 x 6.0 SHS
DS	2/90 x 45 MGP10 PINE STUDS - NAIL LAMINATED
TS	3/90 x 45 F17 KDHW or F17 L.V.L STUDS - NAIL LAMINATED
C1	89 x 89 x 6.0 SHS
C2	100 x 100 x 9.0 SHS
SC	75 x 75 x 6.0 SHS - STUB COLUMN
FJ1	POST-STRUT FLOOR JOISTS OR EQUIVALENT TO MANUFACTURERS SPECIFICATIONS
WB1	TYPE B PLYWOOD BRACING - 4.0kN/m CAPACITY MIN IN ACCORDANCE WITH AS1684; TIMBER FRAMING CODE.
WB2	TYPE B STRAP BRACING - 4.0kN CAPACITY MIN IN ACCORDANCE WITH AS1684; TIMBER FRAMING CODE.

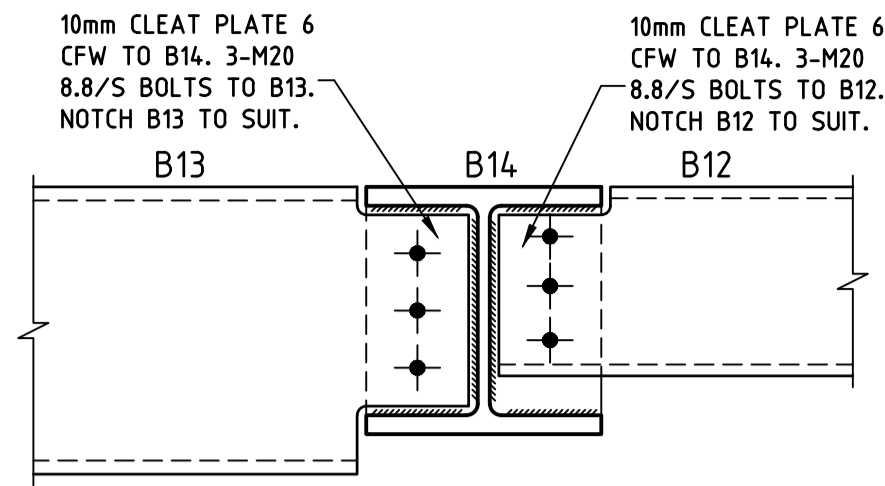
C1	ISSUED FOR CONSTRUCTION	ND	07/02/24	CLIENT:	MR AND MRS VARGHESE
P2	PRELIMINARY 2	ND	02/02/24	ARCHITECT:	ARKI DESIGN STUDIO TEL: 03 9989 5118 EMAIL: PETER@ARKIDESIGN.COM.AU
P1	PRELIMINARY 1	ND	06/12/23		
REVISION	DESCRIPTION	DRAWN	DATE		
				DSL CONSULTING PTY LTD Consulting Structural Engineers A.B.N: 85 611 336 291 659 High Street Kew East, VIC 3102 T: 03 9859 1611 E: structures@dslconsulting.com.au W: www.dslconsulting.com.au	
PROJECT: PROPOSED DUAL OCCUPANCY AT 32 KENDALL STREET HAMPTON				SCALE AT: 1:100 DATE: NOV. 2023 DESIGNED: DAN LAU DRAWN: ND DRAWING NO: 4363-23 REVISION: S3 of 4 C1	



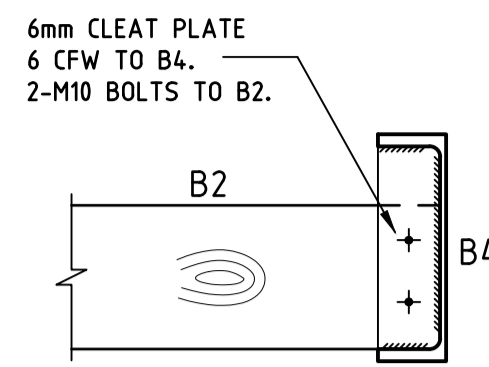
BEAM DETAIL - B3 TO 2R1
SIMILAR FOR 2R1 TO B1 EXCEPT USE 3-M10 BOLTS TO B1



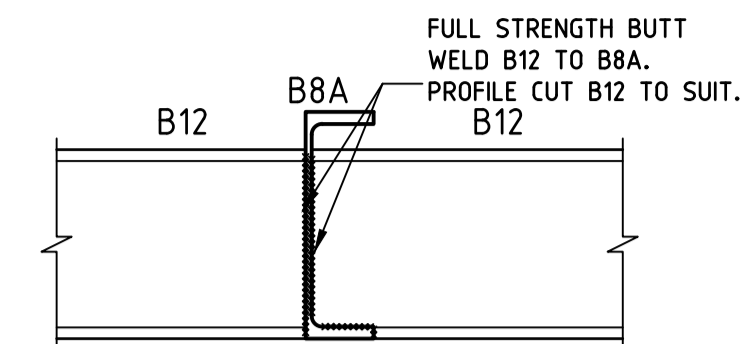
BEAM DETAIL - B12 TO B9
SIMILAR FOR B6 TO B7 EXCEPT USE 3-M20 8.8/S BOLTS
SIMILAR FOR B9 TO B6
SIMILAR FOR B9 TO B7
SIMILAR FOR B7 TO B8 EXCEPT USE 3-M20 8.8/S BOLTS
SIMILAR FOR B8 TO B9
SIMILAR FOR B9 TO B8
SIMILAR FOR B9 TO B9



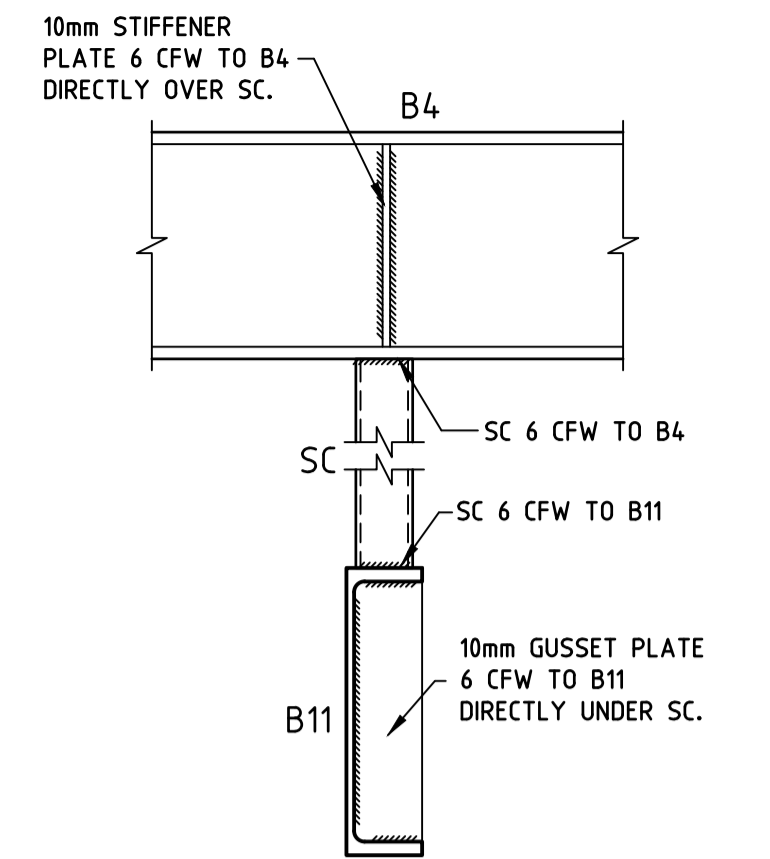
BEAM DETAIL - B13, B12 TO B14



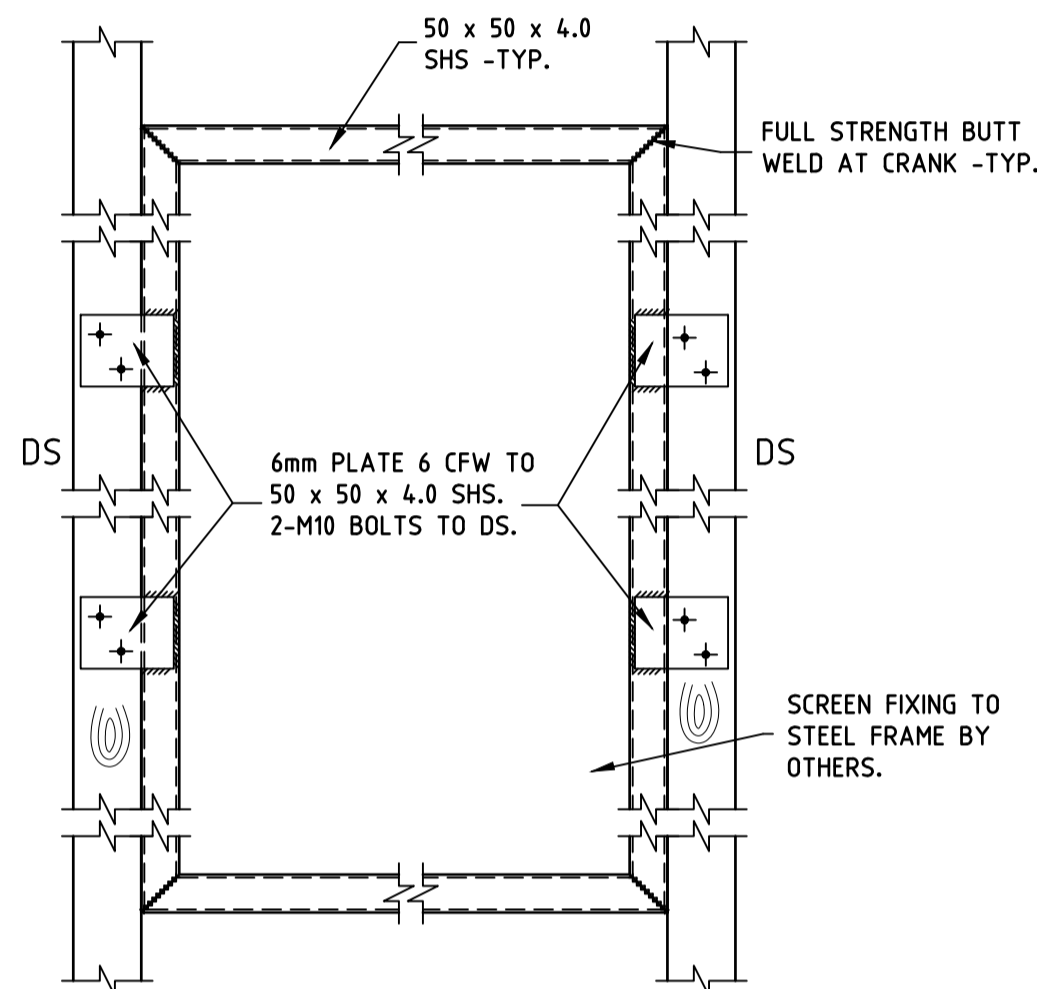
BEAM DETAIL - B2 TO B4
SIMILAR FOR B1 TO B10



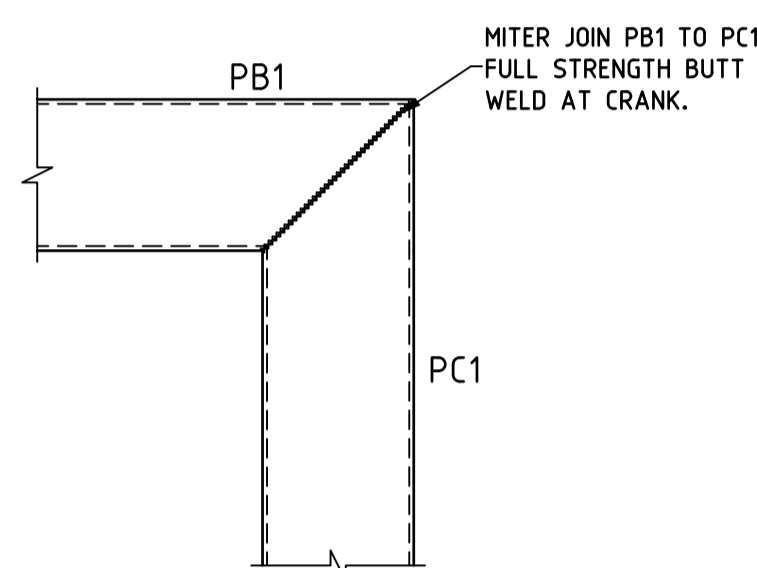
BEAM DETAIL - B12 TO B8A
SIMILAR FOR B9 TO B8
SIMILAR FOR B9 TO B8A
SIMILAR FOR B8 TO B8



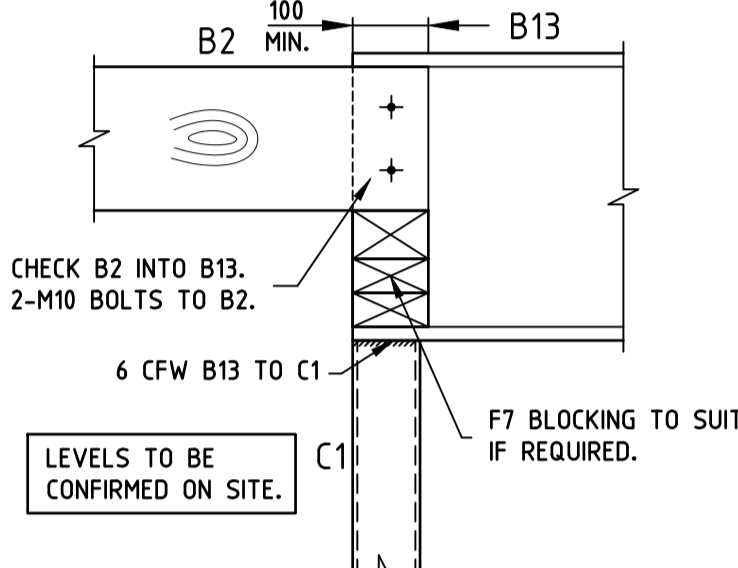
BEAM DETAIL - B4 TO B11



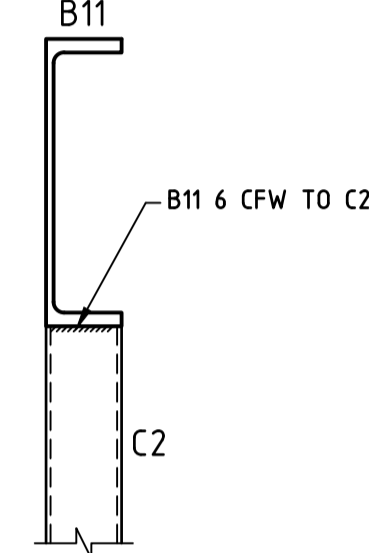
STEEL SCREEN FRAME DETAIL



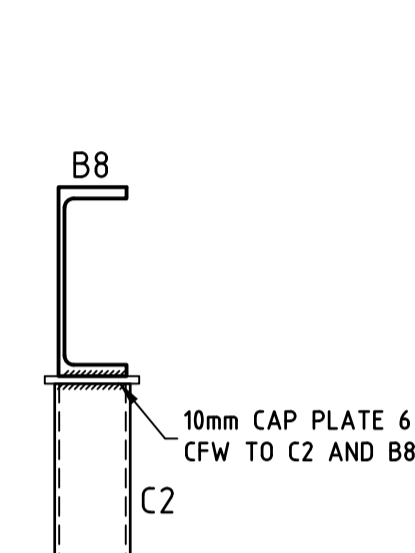
BEAM DETAIL - PB1 TO PC1



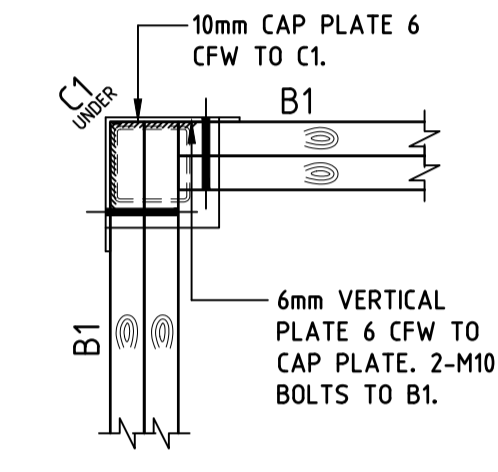
BEAM DETAIL - B2 TO B13
SIMILAR FOR B2 TO B12
SIMILAR FOR B5 TO B4



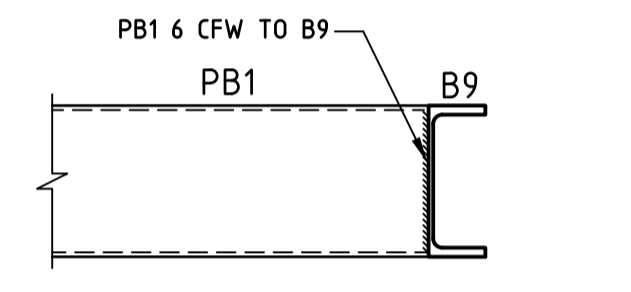
BEAM DETAIL - B11 TO C2
SIMILAR FOR B14 TO C1
SIMILAR FOR B7 TO C1
SIMILAR FOR B4 TO C1



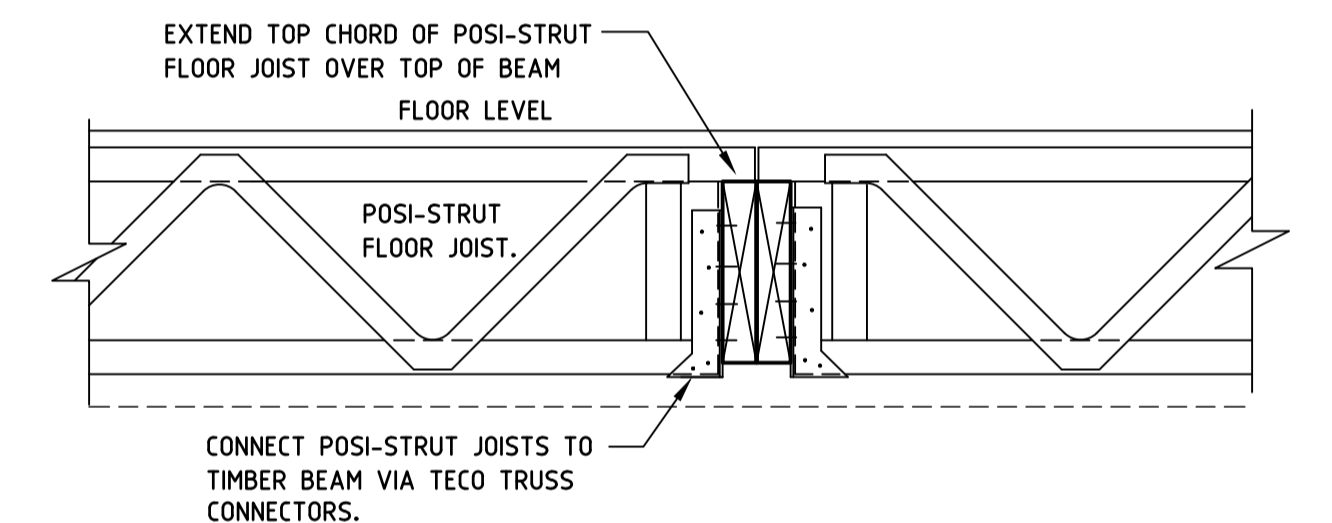
BEAM DETAIL - B8 TO C2
SIMILAR FOR B9 TO C2
SIMILAR FOR B8A TO C2



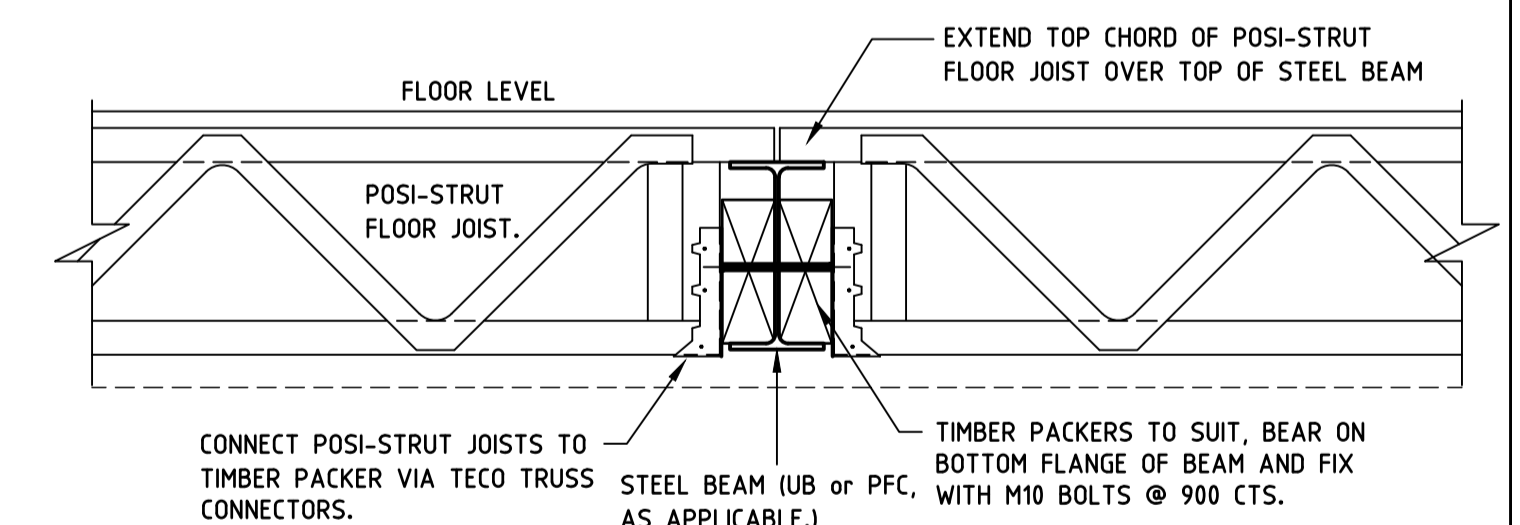
BEAM DETAIL - B1 TO C1



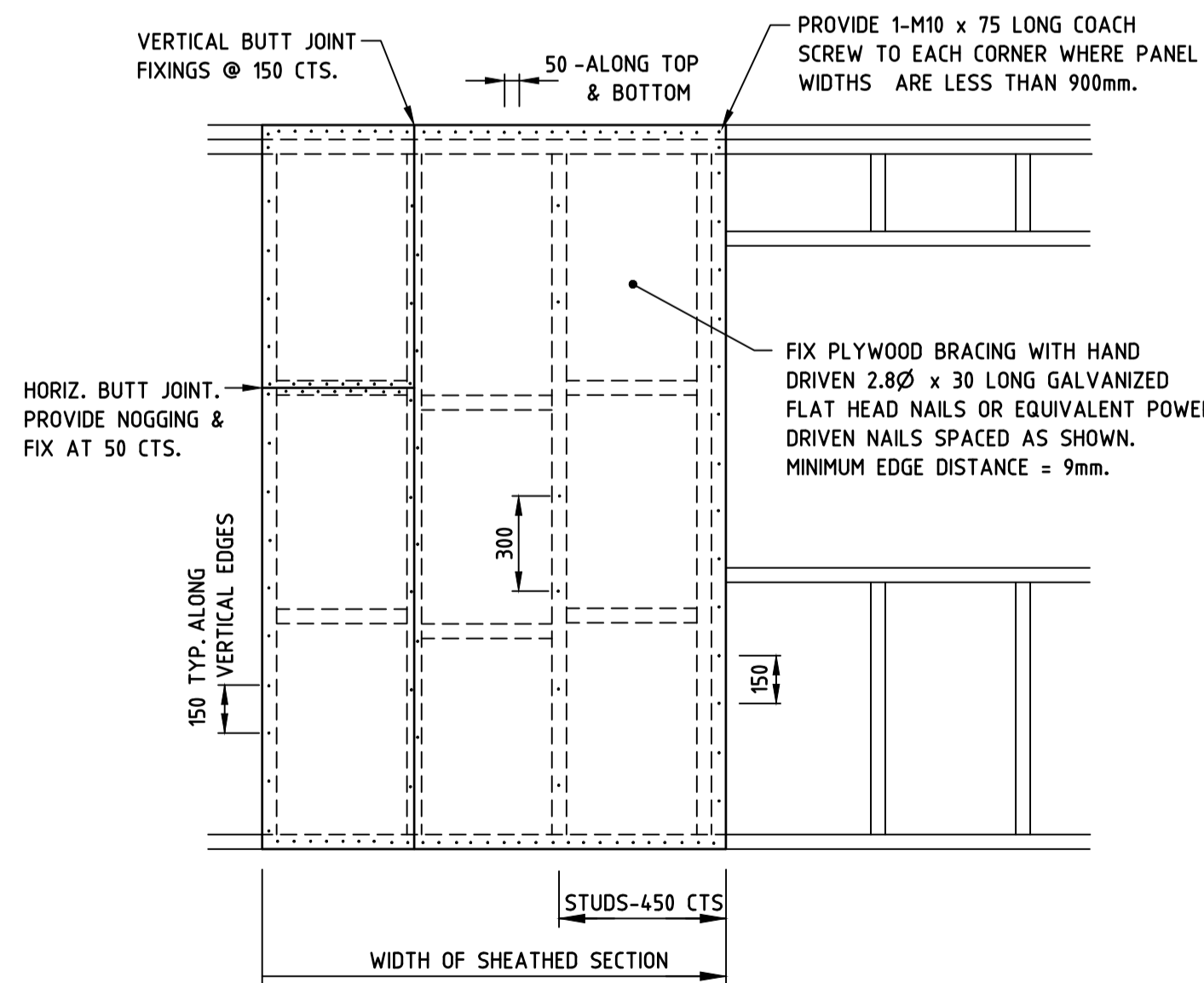
BEAM DETAIL - PB1 TO B9



TYPICAL POSI-STRUT FLOOR JOIST TO TIMBER BEAM
(ALTERNATIVELY, CONNECTION DETAIL TO MANUFACTURERS SPECIFICATIONS)

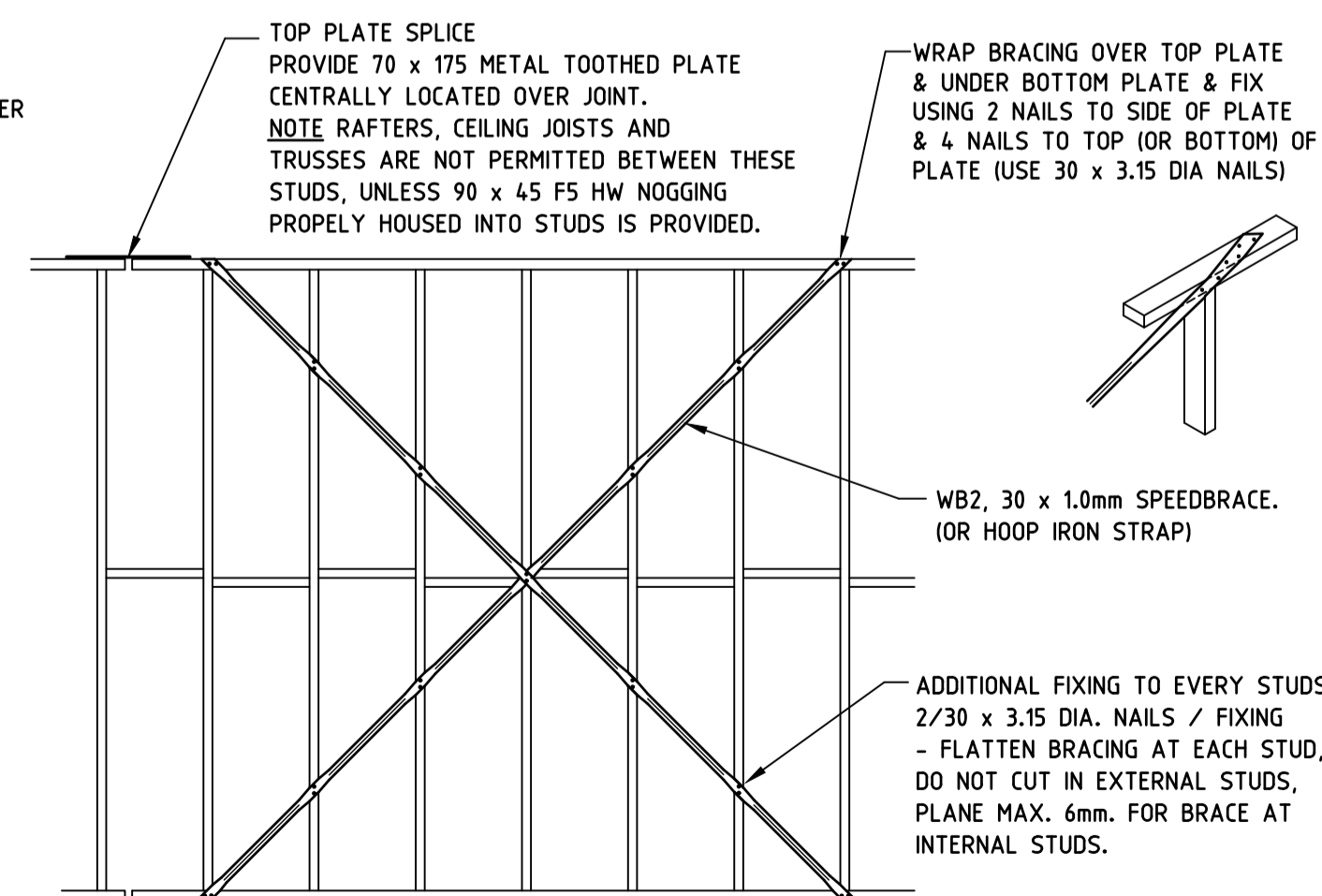


TYPICAL POSI-STRUT FLOOR JOIST TO STEEL BEAM
(ALTERNATIVELY, CONNECTION DETAIL TO MANUFACTURERS SPECIFICATIONS)

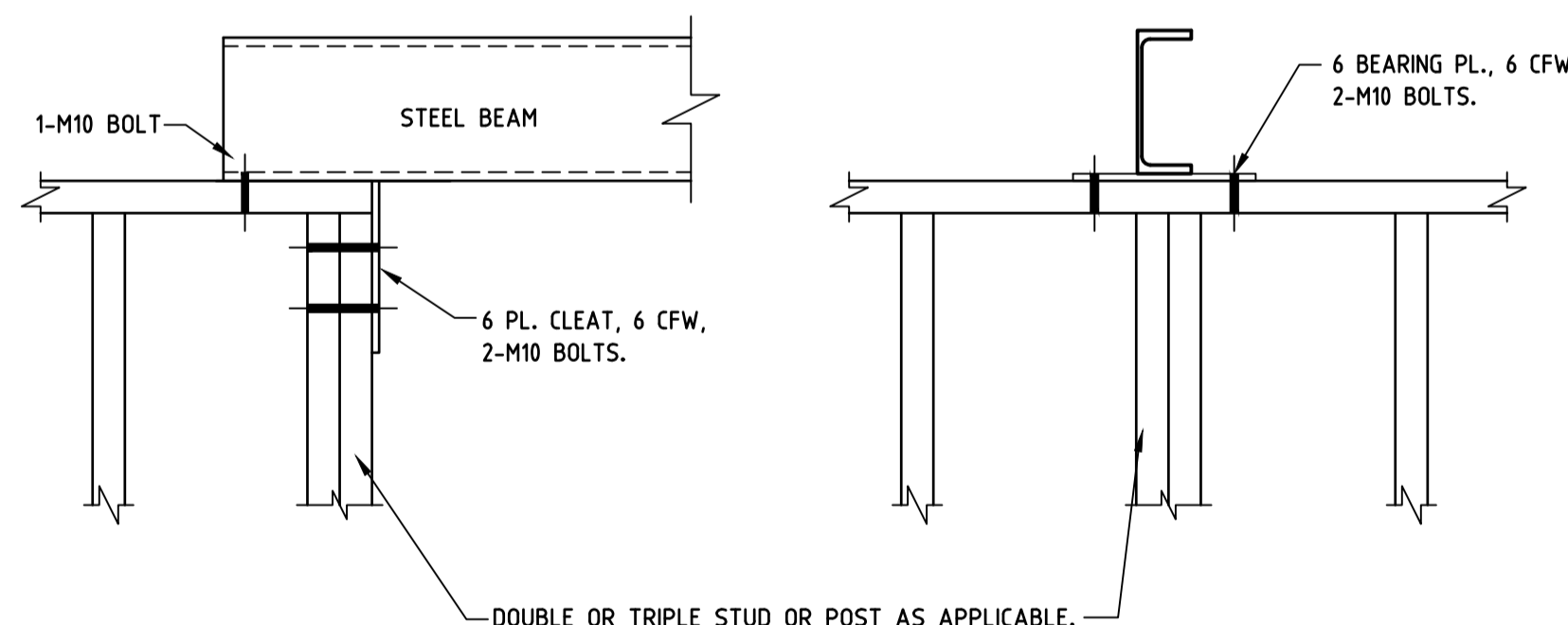


TYPICAL PLYWOOD BRACING DETAIL - WB1 (on plan)
(4.0 kN/m. SAFE WORKING LOAD)
(N.T.S.)

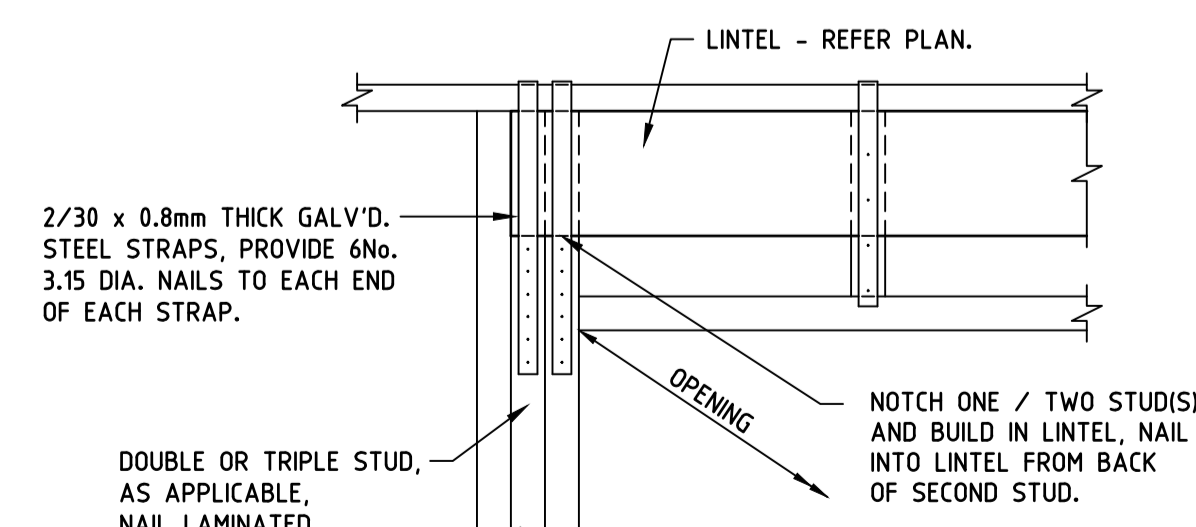
PLYWOOD/STRESS GRADE	THICKNESS
F8	7mm.
F11	6mm.
F14	4mm.



TYPICAL WALL BRACING - WB2 (on plan)
(4.0 kN [TOTAL] SAFE WORKING LOAD)
(N.T.S.)



TYP. STEEL BEAM TO TIMBER POST/STUD DETAILS



TYPICAL TIMBER LINTEL/BEAM DETAIL

REVISION	DESCRIPTION	DRAWN	DATE	CLIENT
C1	ISSUED FOR CONSTRUCTION	ND	07/02/24	MR AND MRS VARGHESE
P2	PRELIMINARY 2	ND	02/02/24	ARCHITECT: ARKI DESIGN STUDIO TEL: 03 9989 5118 EMAIL: PETER@ARKIDESIGN.COM.AU
P1	PRELIMINARY 1	ND	06/12/23	

PROJECT: PROPOSED DUAL OCCUPANCY AT 32 KENDALL STREET HAMPTON			
SCALE AT: 1:10	DATE: NOV. 2023	DESIGNED: DAN LAU	DRAWN: ND
DRAWING NO: 4363-23		S4 of 4	
		C1	



DSL CONSULTING PTY LTD
Consulting Structural Engineers
A.B.N: 85 611 336 291
659 High Street
Kew East, VIC 3102
T: 03 9859 1611
E: structures@dslconsulting.com.au
W: www.dslconsulting.com.au